







	DOOR SCHEDULE					
NO.	QTY.	TYPE	OVERALL SIZE W/H (IN INCHES)	REMARKS	MAX POSITIVE	MAXINEGATIVE
	1	EXT. DBL. SWING	60 X 96		26.1	- 28.4
2	1	GARAGE DR	192 X 96		23.1	- 27.8
3	1	GARAGE DR	108 X 96		25.5	- 32.8
4	1	SIDING GLASS DR.	144 X 96		24.3	- 30.3
5	1	EXT. SWING	36 X 96		23.1	- 27.8
6	5	INT. SWING	32 X 96			
$\widehat{\mathcal{T}}$	8	INT. SWING	30 X 96			
8	1	BI-FOLD	30 X 96			
9						
10						
12						
13						
15						
	VERIFY ALL ROUGH OPENINGS WITH MANUFACTURER PRIOR TO INSTALLATION					

	WINDOW SCHEDULE					
NO.	QTY.	TYPE	OVERALL SIZE W / H (IN INCHES)	REMARKS	MAX POSITIVE PRESSURE	MAX NEGATIVE PRESSURE
A	1	TRANSOM	60 X 24		27.6	- 37.0
B	1	DBL. SINGLE HUNG	74 X 63	EGRESS	25.5	- 32.8
©	4	SINGLE HUNG	37 X 63	EGRESS	26.8	- 35.4
D	2	SINGLE HUNG	37 X 38		27.6	- 37.0
E	1	FIXED GLASS	24 X 30		27.6	- 37.0
F	2	SINGLE HUNG	26 X 38		27.6	- 37.0
G	2	FIXED GLASS	48 X 18		27.6	- 37.0
(H)						
J						
	VERIFY ALL ROUGH OPENINGS WITH MANUFACTURER PRIOR TO INSTALLATION					



TYPICAL BLOCK WALL HURRICANE STRAPPING DETAIL



TYPICAL CEILING EXPOSED TO WIND



1 X 4 OR 2 X 4 DOOR BUCK

BLOOCK WALL -----

STUCCO FININSH

WINDOW HEAD AND JAMB DETAIL

SCALE 3" = 1'

SCALE 1" = 1'

8" 45 DEGREE REINFORCED MASONRY CORNER BLOCK WITH (2) - #5 VER BAR FILLED WITH A MIN. OF 3000 P.S.I. CONCRETE

40 BAR DIAMETERS.

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LINTELS SCHEDULE			
PCL-1	PRECAST LINTEL	8/14 - 1 U/1 L	8F12 - 1T/1B
PCL-2	PRESTRESSED LINTEL	8/16 - 1 U/1 L	8F16 - 1T/1B
PCL-3	PRESTRESSED LINTEL		8F16 - 2T/1B

1/2S	SSSSS SSSSSS	1/2S	
			► W →
	BOTTOM REINF.		TOP REINF.
	SPAN		#3 STIRRUPS d
	-		
			1-1\2" CLEARANCE
	#5 VERTICAL BAR HOOKING INTO TIE BEAM. FILL CELLS WITH 3000		(TYPICAL)
	P.S.I CONCRETE. BARS SPACING ARE NOT TOO EXCEED 4'-0" AND		
	LOCATED EACH SIDE OF ALL WINDOW AND DOOR OPENINGS AND		
	EACH SIDE OF ALL CORNERS.		

HEADER NUMBER

WH-1

			I TRU
	THEADER SCHEDUILE		1.[
WOOI	D HEADER SCHEDOLE		н
SIZE	END ATTACH MENT	DETAIL	
(2)-2x10 No. 2 SYP AND (1) LAYERS OF 1/2" PLYWOOD	(1)-STUD WITH (1)-SIMPSON MSTA24 OR EQUAL	M	
			AME
			REG

GANG STUD COLUMN SCHEDULE					
COLUMN I.D.	STUDS	BASE ATTACHMENT	TRUSS/HEADER ATTACHMENT	NOTE	
GS-1	(3) PLY 2 X 4	SIMPSON HTT4(a)	(2) SIMPSON HTS20 OR (2) MSTA24 AS REQUIRED	-	
(a) ATTACH SIMPSON HTT4 WITH 5/8" DIA. TITEN HD WITH 6" MIN. EMBEDMENT					

ELECTRICAL NOTES:

1. ELECTRICAL PLAN HAS BEEN PREPARED IN COMPLIANCE WITH THE FLORIDA BUILDING CODE 2020, 7th EDITION AND 2014 NATIONAL ELECTRICAL CODE. THE ELECTRICAL CONTRACTOR SHALL COMPLY WITH ALL FLORIDA BUILDING CODES AS WELL AS ANY LOCAL CODE THAT MAY BE APPLICABLE.

2. ALL BRANCH CIRCUITS THAT SUPPLY 120 VOLT, SINGLE PHASE, 15-20 AMPERE OUTLETS INSTALLED INDWELLING UNITS, FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, DENS, BEDROOMS, CLOSETS, HALLWAYS, SUNROOMS, OR ANY OTHER SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE, INSTALLED TO PROVIDE PROTECTION TO THE BRANCH CIRCUIT.

3. SMOKE DETECTORS SHALL BE HARDWIRED AND INTERCONNECTED WITH BATTERY BACKUP

GENERAL NOTES

- 1. The contractor or subcontractor shall verify all conditions and dimensions at the job site prior to commencing work. The contractor shall report all discrepancies between the drawing and existing conditions to the designer prior to commencing work.
- 2. General contractor reserves the right to adjust dimensions as may be required at his discretion, due to field conditions of for any reason.
- 3. All details and sections shown on the drawings are intended to be typical and shall be construed to apply to any similar situation elsewhere in the work except where a different detail is shown.
- 4. All trusses shall be designed and certified by the truss manufacturer's state of Florida registered engineer.
- 5. Refer to boundry survey completed by professional land surveyors for existing conditions and grade elevations.
- 6. The structure is designed to be self supporting and stable after the building is complete. It is the contractor's sole responsibility to determine erection procedures and sequence to ensure safety of the building and its components during erection. This includes the addition of necessary shoring, sheeting, temporary, bracing, guys or tie downs.
- 7. Ruben Leon Designs LLC Assumes no liability for location of the structure on the site.
- 8. If any errors or omissions exist in the drawings and specifications, the contractor shall within fifteen days after receiving of this plans and prior to construction, notify Ruben Leon Designs LLC of such errors or omissions or be held responsible for the results and cost.
- 9. These documents are the property of Ruben Leon Designs LLC and may not be used or reproduced without expressed written consent.

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R WALL WHERE "PAN" FLASHING IS USED AT THE SILL, ALSO INCORPORATE FLASHING OP	not provided by the window or door manufacturer or by the flashing manufacturer, "pan flashing" shall be installed at the sill of exterior window and door openings. Pan flashing shall be sealed or sloped in such a manner as to direct water to the surface of the exterior wall finish or to the water-resistive barrier for subsequent drainage. Openings using pan flashing shall also incorporate flashing or protection at the head and sides.
PROTECTION AT THE HEAD AND SIDES	^{II} Pan Flashing ^{II} is a generic term that used to refer to "metal pan flashing". However many modern materials have been developed for the same function such as: - Flexible Peel n Stick Flashing Membrane - Fluid Applied Flashing For such products, follow the manufacturer's installation instructions.
TALL "PAN" FLASHING THE SILL	For in-depth flashing installation instructions, refer to the following publications: FMA/AAMA 100 FMA/AAMA 200 FMA/WDMA 250 FMA/AAMA/WDMA 300
IONS FROM THE WIND UPERCEDE THIS DETA	OW / DOOR MFR., OR THE IL.

ELECTRICAL PLAN SCALE 1/4" = 1

STRUCTURAL NOTES:

GENERAL NOTES:

COORDINATION OF CONSTRUCTION INCLUDING VERIFICATION OF DIMENSIONS, ELEVATIONS, AND FIELD CONDITIONS IS THE RESPONSIBILITY OF THE CONTRACTOR. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO CONSTRUCTION. SEE ARCHITECTURAL DRAWINGS FOR DETAILS AND DIMENSIONS NOT SHOWN.

INFORMATION PERTAINING TO THE EXISTING CONDITIONS GIVEN ON THESE STRUCTURAL DRAWINGS REPRESENTS TO THE BEST OF OUR KNOWLEDGE THE ACTUAL EXISTING FIELD CONDITIONS. AMERICAN STRUCTURAL ENGINEERING, INC. MAKES NO WARRANTY AS TO THEIR ACCURACY. THE CONTRACTOR SHALL FIELD VERIFY THE EXISTING CONDITIONS IMPERATIVE TO THE NEW WORK AND REPORT DISCREPANCIES BETWEEN THE DRAWINGS AND THE FIELD CONDITIONS TO THE ENGINEER FOR REVIEW.

DRAWINGS SHALL NOT BE SCALED. CONTACT THE ENGINEER OR ARCHITECT IF CLARIFICATION OF ANY DIMENSION IS REQUIRED.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE PROPER ERECTION PROCEDURES TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION; THIS INCLUDES THE INSTALLATION OF ALL SHORING, RE-SHORING, BRACING, TIE DOWNS, ETC.

IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY FOR THE SAFETY OF THE WORKERS DURING ALL PHASES OF CONSTRUCTION AND FOR ADHERENCE TO ALL APPLICABLE LAWS AND REGULATIONS GOVERNING ON-SITE SAFETY PROCEDURES.

DESIGN LOADS:

THE STRUCTURAL SYSTEMS FOR THE ATTACHED DRAWINGS WERE DESIGNED PER THE WIND LOADING PRESENTED IN SECTION 1609 OF THE FLORIDA BUILDING CODE 2020, 7th EDITION (BUILDING) AND GRAVITY LOADING PRESENTED IN SECTION R301 OF THE FLORIDA BUILDING CODE 2020, 7th EDITION (RESIDENTIAL) USING THE FOLLOWING SUPERIMPOSED LOADS:

FLOOR (LIVE LOAD): TYPICAL	40 PSF 40 PSF 60 PSF 80 PSF 50 PSF
ROOF: TOP CHORD LIVE LOAD TOP CHORD DEAD LOAD BOTTOM CHORD LIVE LOAD BOTTOM CHORD DEAD LOAD ATTIC LIVE LOAD	
SCREEN ENCLOSURE	14 PSF
LATERAL EARTH PRESSURE	45 PSF/FT

WIND LOAD:

SEE "WIND LOAD CRITERIA" AT RIGHT

PROTECTION OF OPENINGS:

ALL WINDOWS AND DOORS SHALL BE DESIGNED BY THE MANUFACTURER PER THE WIND LOAD CRITERIA SHOWN ON THIS SHEET. CERTIFICATIONS SHALL BE PROVIDED TO THE BUILDING DEPARTMENT FOR VERIFICATION OF COMPLIANCE WITH SECTION 1609.1.2, "PROTECTION OF OPENINGS", OF THE FLORIDA BUILDING CODE 2020, 7th EDITION (BUILDING), ALL GLAZED OPENINGS SHALL HAVE PROTECTION FROM WINDBORNE DEBRIS.

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DOCUMENTS ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY COMPLIANCE WITH THE STRUCTURAL DOCUMENTS AS IT PERTAINS TO THE QUANTITY, LENGTH, DIMENSIONS, ETC.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT OR ENGINEER FOR REVIEW, OR THE DRAWINGS WILL BE RETURNED UNCHECKED.

SHOP DRAWINGS MARKED "REJECTED" OR "AMEND AND RESUBMIT" SHALL BE CORRECTED BY THE SUPPLIER AND RESUBMITTED TO THE ENGINEER OF RECORD FOR ADDITIONAL REVIEW.

THE STRUCTURAL DOCUMENTS SHALL SUPERSEDE THE SHOP DRAWINGS IN ALL CASES UNLESS NOTED IN WRITING BY THE ENGINEER OF RECORD.

SHOP DRAWINGS- SPECIALTY ENGINEERING:

NECESSARY FOR PROPER FABRICATION AND INSTALLATION.

THE FOLLOWING ITEMS REQUIRE FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER:

WOOD TRUSSES
 PRECAST COMPONENTS (PLANKS, BEAMS, LINTELS, ETC.)

SUBMITTALS SHALL IDENTIFY THE PROJECT, APPLICABLE CODES AND THE DESIGN CRITERIA. SUBMITTALS SHALL ALSO SHOW ALL DETAILS AND PLANS

SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER.

ENGINEERED SHOP DRAWINGS AND CALCULATIONS REQUIRE THE DATE, SEAL, AND SIGNATURE OF THE DELEGATED ENGINEER. IF COMPUTER GENERATED CALCULATIONS ARE PROVIDED, THEY SHALL BEAR THE SEAL AND SIGNATURE OF THE DELEGATED ENGINEER TO ACKNOWLEDGE ACCEPTANCE AND RESPONSIBILITY FOR THE RESULTS. THE ENGINEER OF RECORD SHALL RETAIN ONE COPY OF THE SHOP DRAWINGS FOR RECORD.

DRAWINGS PREPARED SOLELY AS A GUIDE FOR FABRICATION AND ERECTION DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER. EXAMPLES OF SUCH DRAWINGS INCLUDE REINFORCING STEEL AND STRUCTURAL STEEL ERECTION DRAWINGS.

CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

SHOP DRAWING REVIEW BY THE ENGINEER OF RECORD IS LIMITED TO VERIFYING THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT, AND HAS USED THE CRITERIA SPECIFIED ON THE STRUCTURAL DOCUMENTS IN THE PREPARATION OF THE SHOP DRAWINGS; ALSO THAT THE CONFIGURATION DEPICTED IN THE SHOP DRAWINGS IS CONSISTENT WITH THE STRUCTURAL DOCUMENTS. NO DETAILED CHECK OF DIMENSIONS, QUANTITIES OR CALCULATIONS WILL BE MADE. FORMWORK AND SHORING:

FORM WORK SUPPORTING STRUCTURAL CONCRETE (BEAMS, SLABS, ETC.) MAY NOT BE REMOVED UNTIL THE CONCRETE HAS ATTAINED 75% OF THE 28 DAY DESIGN MINIMUM STRENGTH. DETERMINATION OF THE IN PLACE CONCRETE STRENGTH SHALL BE DETERMINED BY LABORATORY TESTING OF CONCRETE CYLINDERS.

FORMS SHALL BE CLEAN FROM DEBRIS PRIOR TO PLACEMENT OF CONCRETE.

FILL:

IF NOT OTHERWISE SPECIFIED, ALL FILL SHALL BE CLEAN COARSE SAND, FREE OF ROOTS AND OTHER DELETERIOUS OR ORGANIC MATERIAL. FILL SHALL BE PLACED IN MAXIMUM 12" LIFTS AND COMPACTED WITH A HEAVY VIBRATORY ROLLER TO A MINIMUM 95% OF MAXIMUM MODIFIED PROCTOR DENSITY IN ACCORDANCE WITH ASTM D1557.

IF FILL IS LESS THAN 18" DEEP, DENSITY TESTS SHALL BE PROVIDED FOR THE FULL DEPTH TAKEN AT FINISH GRADE. IF FILL IS GREATER THAN 18" DEEP, DENSITY TESTS SHALL BE PROVIDED FOR EACH 12" LIFT.

ALLOWABLE SOIL BEARING CAPACITY SHALL BE 2,000 PSF, (ASSUMED)

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE SOIL CONDITIONS PRIOR TO CONSTRUCTION AND REPORT ANY ANOMALIES TO THE ENGINEER OF RECORD.

CONCRETE:

ALL CONCRETE SHALL BE PLACED IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI-318).

THE FOLLOWING MINIMUM DESIGN STRESSES SHALL BE MET:

CAST IN PLACE CONCRETE:

FOOTINGS f'c =	3000 PSI
COLUMNS/PILASTERS fc =	3000 PSI
SLAB ON GRADE fc =	3000 PSI
BEAMS, ELEVATED SLABS fc =	3000 PSI
GROUTED MASONRY CELLS, OTHER fc =	3000 PSI
REINFORCEMENT:	
TIES, STIRRUPS fy =	60,000 PSI
WELDED WIRE FABRIC fv =	60.000 PSI
ALL OTHER REINFORCEMENT fy =	60,000 PSI
STRUCTURAL STEEL & EMBEDS:	
BOLTS_ASTM A325 Ev =	36.000 PSI
STEEL PLATES, ASTM A36	36,000 PSI

THE CONCRETE STRESSES LISTED ABOVE ARE BASED ON A 28 DAY COMPRESSIVE STRENGTH AS DETERMINED BY LABORATORY TESTING OF CONCRETE CYLINDERS.

REINFORCING STEEL SHALL BE ASTM A615, GRADE 60 DEFORMED BARS FREE FROM OIL, SCALE AND RUST. WELDED WIRE FABRIC SHALL BE ASTM A185 IN FLAT MANUFACTURED SHEETS.

CONCRETE SHALL BE PLACED AND CURED TO ACI STANDARDS AND SPECIFICATIONS.

PROPOSED DESIGN MIXES SHALL BE PROVIDED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. DESIGN MIXES SHALL INCLUDE RECENT FIELD CYLINDERS OR LAB TESTS. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED TO THE TIME OF PLACEMENT IN ITS FINAL POSITION SHALL NOT EXCEED °90 MINUTES.° IF FOR ANY REASON THERE IS A LONGER DELAY, THE CONCRETE SHALL BE DISCARDED. NO WATER SHALL BE ADDED ON SITE.

ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM C309, TYPE 1-D. THE COMPOUND SHALL BE PLACED IMMEDIATELY AFTER FINISHING. CALCIUM CHLORIDES SHALL NOT BE USED THE CONTRACTOR SHALL CONFIRM THE COMPATIBILITY OF THE CURING COMPOUND WITH FLOOR FINISHES.

REINFORCING STEEL CLEAR COVER REQUIREMENTS SHALL BE AS FOLLOWS:

PERMANENTLY EXPOSED TO EARTH ______ 3"
CONCRETE EXPOSED TO EARTH OR WEATHER:

CONCRETE NOT EXPOSED TO EARTH OR WEATHER OR IN CONTACT WITH GROUND:

CONCRETE CAST AGAINST FARTH AND

UP THRU #11 BARS #14 AND #18 BARS	
BEAMS	1 1/2
COLUMNS	1.1/2

ALL REINFORCING BARS SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETERS IN CONCRETE UNLESS NOTED OTHERWISE.

ALL WELDED WIRE FABRIC SHALL BE LAPPED A MINIMUM OF ONE SPACE. WELDED WIRE FABRIC SHALL BE SUPPORTED AT A MAXIMUM OF 3 FEET BETWEEN SUPPORTS AND SHALL BE LOCATED IN THE BOTTOM HALF OF THE SLAB, A MINIMUM OF 1 1/2" OFF THE BOTTOM. THE VAPOR BARRIER SHALL BE MIN. 6 MIL AND SHALL BE LAPPED 6". ALL JOINTS MUST BE TAPED.

ALL FOOTING DOWELS SHALL MATCH THE SIZE AND SPACING OF THE VERTICAL REINFORCEMENT. TYPICAL #5 DOWELS OR VERT. BARS EMBEDDED INTO THE CONCRETE FOUNDATION SHALL HAVE A 10"-90± BEND UNLESS NOTED OTHERWISE, AND SHALL BE EMBEDDED A MINIMUM OF 7".

ALL LONGITUDINAL BARS WITHIN FOOTINGS, WALLS, SLABS, BEAMS, ETC. SHALL BE CONTINUOUS UNLESS NOTED OTHERWISE.

ALL REINFORCING BARS SHALL BE TIED IN PLACE WITH THE PROPER COVER PRIOR TO ANY PLACEMENT OF CONCRETE PER ACI-318. "WET STICKING" OF THE REINFORCEMENT AFTER PLACEMENT OF THE CONCRETE WILL NOT BE ALLOWED.

CORNER REINFORCEMENT SHALL BE PROVIDED AT ALL INTERSECTIONS OF CONCRETE BEAMS AND FOOTINGS. BARS SHALL BE PLACED AT TOP AND BOTTOM AND SHALL MATCH THE LARGEST TERMINATING REINFORCING BAR.

ALL STRUCTURAL CONCRETE SHALL BE CONSOLIDATED BY INTERNAL VIBRATION.

MASONRY WALLS:

ALL MASONRY CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ACI-530/ASCE-5/TMS 402, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", LATEST EDITION.

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY UNIT. MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270.

ALL MASONRY SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH (fm) OF 1500 PSI. GROUTED MASONRY CELLS SHALL BE FILLED WITH A GROUT THAT ACHIEVES A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS.

ALL WALLS SHALL BE CONSTRUCTED WITH RUNNING BOND.

ALL REINFORCING BARS SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETERS IN GROUTED MASONRY UNLESS NOTED OTHERWISE.

SEE STRUCTURAL PLANS FOR THE LOCATION OF ALL VERTICAL REINFORCEMENT. HORIZONTAL MASONRY WALL REINFORCING SHALL BE CONTINUOUS STEEL HORIZONTAL REINFORCEMENT, INCLUDING ALL CORNERS AND INTERSECTIONS AND SHALL BE PROVIDED FOR ALL MASONRY WALLS 10'-0" TALL OR GREATER OR WHERE NOTED ON THE PLANS. PROVIDE A 2-WIRE, 9 GAUGE (0.1495") LADDER TYPE HORIZONTAL REINFORCEMENT AT EVERY OTHER COURSE (16" O.C.).

SUBMIT PROPOSED GROUT MIX DESIGN TO ENGINEER OF RECORD PRIOR TO USE. USE OF CONCRETE FOR FILLED CELLS IS PROHIBITED.

CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF ALL CELLS TO BE FILLED WITH GROUT IN EXCESS OF 24" HIGH. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING AND AFTER INSPECTION. CELLS SHALL BE FREE OF DEBRIS, OVERHANGING MORTAR, OR OTHER OBSTRUCTIONS PRIOR TO GROUTING.

VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 10'-0". GROUT SHALL BE POURED IN MAXIMUM 4'-0" LIFTS AND SHALL BE CONSOLIDATED BY VIBRATION. EACH SUBSEQUENT LIFT SHALL BE POURED AND VIBRATED BEFORE PLASTICITY IS LOST.

IF GROUTING IS STOPPED FOR 60 MINUTES OR GREATER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE GROUT POUR A MINIMUM OF 1 1/2" BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.

A MAXIMUM #5 BAR MAY BE PROVIDED WITHIN AN 8" WIDE MASONRY WALL (NOMINAL WIDTH). LARGER BARS WILL NOT BE PERMITTED UNLESS NOTED OTHERWISE

ALL INTERIOR GRADE BEAMS OF A PILE SUPPORTED FOUNDATION MEETING A MASONRY STEM WALL REQUIRE THE MASONRY TO BE BROKEN OUT TO RECEIVE THE BEAM. THE REINFORCEMENT OF THE GRADE BEAM SHALL HOOK INTO THE MASONRY CELL WITH A 6" MINIMUM HOOK.

LINTELS:

IF PRECAST CONCRETE LINTELS ARE PROVIDED, EACH LINTEL MUST BE A MINIMUM OF 8"X8" WITH (1) #5 BAR TOP AND BOTTOM (8F8-1T/1B) AND SHALL BE FILLED SOLID WITH MINIMUM 3000 PSI GROUT. ALL LINTELS SHALL HAVE MINIMUM 8" BEARING AT EACH END. PRECAST CONCRETE LINTELS MAY NOT BE USED IF ABUTTING A CAST IN PLACE CONCRETE COLUMN. LINTELS MAY NOT BE USED ABOVE OPENINGS GREATER THAN 12'-0".

VERTICAL REINFORCEMENT INTERRUPTED BY AN OPENING SHALL BE PROVIDED AT 4'-0" O.C. FROM THE LINTEL TO THE TIE BEAM. ALL VERTICAL BARS MUST HOOK INTO THE LINTEL WITH A MINIMUM 8" HOOK. GROUT THE CELLS CONTAINING VERTICAL STEEL SOLID AS DESCRIBED UNDER "MASONRY WALLS" ABOVE.

STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AISC "MANUAL OF STEEL CONSTRUCTION" WITH THE FOLLOWING PROPERTIES:

TUBE SECTIONS (ASTM A-500, GR, B)	———— Fv =	46.000 PSI
WIDE FLANGE BEAMS (GR. 50)	————— Fy =	50,000 PSI
CHANNELS, MISC. SHAPES (ASTM A-36)	———— Fy =	36,000 PSI
PIPE COLUMNS	———— Fy =	36,000 PSI
BOLTS, ASTM A325	——————————————————————————————————————	36,000 PSI

ALL WELDED CONNECTIONS SHALL UTILIZE THE E70xx ELECTRODE. ALL SHOP CONNECTIONS SHALL BE WELDED AND ALL FIELD CONNECTIONS SHALL BE MADE AS SPECIFIED ON THE STRUCTURAL PLANS. IN NO CASE SHALL ANY BOLTED CONNECTION HAVE LESS THAN (2) ROWS OF BOLTS.

ALL STRUCTURAL STEEL USED AS AN EXTERIOR APPLICATION (WHETHER WRAPPED WITH A FINISH MATERIAL OR LEFT EXPOSED) SHALL BE HOT-DIPPED GALVANIZED (G90). ALL OTHER STRUCTURAL STEEL SHALL BE SHOP PRIMED WITH RED OXIDE PAINT.

FASTENERS AND MECHANICAL CONNECTORS SHALL BE GALVANIZED.
STRUCTURES EXPOSED TO WEATHER OR IN EXPOSURE CATEGORY "C" AS
DEFINED IN THE FLORIDA BUILDING CODE SHALL HAVE FASTENERS EITHER:
(A) - HOT-DIPPED GALVANIZED WITH 1.5 OZ. ZINC PER 1 SQ.FT.
(B) - TRIPLE ZINC COATED PER ASTM A90.
(C) - STAINLESS STEEL

INSTALL ALL MECHANICAL CONNECTORS AS SPECIFIED IN THE MANUFACTURER'S CATALOG. CONNECTORS SHALL BE "SIMPSON STRONG TIE" UNLESS NOTED OTHERWISE

SHEAR STUDS:

SHEAR STUDS OR HEADED STUDS SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE", SECTION 7-STUD WELDING. STUDS SHALL BE TYPE "B" HEADED STUDS HAVING A MINIMUM TENSILE STRENGTH OF 60,000 PSI AND SHALL BE OF LENGTH, DIAMETER AND CONFIGURATION AS SHOWN ON THE STRUCTURAL DOCUMENTS.

WOOD:

THE STRUCTURAL WOOD DESIGN SHOWN ON THE DRAWINGS HAS BEEN PERFORMED IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" (NDS), 2012.

STRUCTURAL WOOD COMPONENTS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES AT NORMAL LOAD DURATION UNDER DRY SERVICE OF CONDITION:

BEAMS/HE	EADERS (#2 S.Y.P.)
Fb =	1100 PSI
Fv =	90 PSI
Fc_ 1	480 PSI
E =	1,400,000 PSI

2X4 & 2X6 STUDS (#2 S.Y.P.):

E = 1,400,000 PSI

Fb = 1150 PSI

Fc **=I** 1500 PSI

LVL'S:

6X6 AND LARGER POST (#2 S.Y.P.): Fb = 850 PSI Fc **1** 525 PSI E = 1,200,000 PSI

ALL STRUCTURAL LUMBER SHALL BE #2 SOUTHERN YELLOW PINE (S.Y.P.) OR BETTER UNLESS NOTED OTHERWISE

ALL WOOD IN CONTACT WITH CONCRETE OR EARTH, OR EXPOSED TO WEATHER SHALL BE PRESSURE TREATED (P.T.) UNLESS AN APPROVED MOISTURE BARRIER IS PROVIDED.

ALL WOOD FRAMING WITHIN 8" OF EXPOSED EARTH SHALL BE PRESSURE TREATED (P.T.).

THERE SHALL BE A MINIMUM OF 6" BETWEEN ANY WOOD SIDING, WALL SHEATHING OR FRAMING MEMBERS AND EXPOSED EARTH.

PLYWOOD SPECIFIED ON THE STRUCTURAL DRAWINGS SHALL COMPLY WITH ALL PROVISIONS OF CHAPTER 23 OF THE FLORIDA BUILDING CODE, 5th EDITION (BUILDING).

PLYWOOD ROOF AND FLOOR SHEATHING SHALL BE INSTALLED WITH THE LONG DIMENSION PERPENDICULAR TO THE FRAMING MEMBERS. JOINTS SHALL BE STAGGERED.

SEE STRUCTURAL DETAILS ELSEWHERE FOR THE SIZE AND SPACING OF THE REQUIRED PLYWOOD FASTENERS. ROOF SHEATHING MUST BE FASTENED WITH MINIMUM 8d RING SHANK NAILS.

ALL SAWN LUMBER SIZES NOTED ON THE STRUCTURAL DRAWINGS ARE NOMINAL SIZES. ALL LVL SIZES NOTED ON THE DRAWINGS ARE ACTUAL DIMENSIONS.

P.T. LUMBER REQUIREMENTS:

THE FOLLOWING REQUIREMENTS APPLY TO ACQ (ALKALINE COPPER QUATERNARY) TREATED LUMBER. THE STRUCTURAL PLANS INDICATE "P.T." WHEREVER PRESSURE TREATED LUMBER IS REQUIRED. PRESSURE TREATED LUMBER SHALL BE PROVIDED AT ALL LOCATIONS WHERE LUMBER IS SUBJECT TO WEATHERING OR WHERE THERE IS A LIKELIHOOD OF PROLONGED EXPOSURE TO MOISTURE.

FASTENERS:

- 1. ALL FASTENERS USED TO JOIN P.T. LUMBER (e.g. BUILT-UP MULTI PLY BEAM) OR FASTEN INTO P.T. LUMBER SHALL BE ONE OF THE FOLLOWING:
- A. HOT DIPPED GALVANIZED.
- B. STAINLESS STEEL (304 OR 316).C. CERAMIC COATED (COATING APPROVED FOR USE WITH ACQ
- LUMBER). D. "Z-MAX" (G-185) BY "SIMPSON STRONG-TIE CO., INC." OR EQUIVALENT METHOD OF APPLYING 1.85 OZ. OF ZINC PER SQUARE
- FOOT OF SURFACE AREA TO THE FASTENER.

2. ALL FASTENERS MUST BE LABELED AS APPROVED FOR USE WITH ACQ PRESSURE TREATED LUMBER.

 P.T. BUCKING SHALL BE FASTENED TO MASONRY OR CONCRETE WITH STAINLESS STEEL (304 OR 316) OR CERAMIC COATED MASONRY SCREWS (e.g. "TAPCONS"). ALL WINDOW AND DOOR UNITS FASTENED INTO THE BUCKING ONLY SHALL BE ATTACHED WITH STAINLESS STEEL SCREWS.

STEEL:

1. UNPROTECTED STEEL SHALL NOT BE USED WITH PRESSURE TREATED LUMBER. STEEL SHALL BE PROTECTED WITH ONE OF THE FOLLOWING:

A. HOT DIPPED GALVANIZED.B. STAINLESS STEEL (304 OR 316).

C. PAINTED WITH A PROTECTIVE COATING TO PROVIDE A PHYSICAL BARRIER BETWEEN STEEL AND P.T. LUMBER.

ALUMINUM:

1. ALUMINUM SHALL NOT COME INTO CONTACT WITH PRESSURE TREATED LUMBER. THIS APPLIES FOR ANY GRADE OF ALUMINUM (STRUCTURAL OR NON-STRUCTURAL). ONE OF THE FOLLOWING METHODS SHALL BE USED WHEN FASTENING ALUMINUM TO P.T. LUMBER:

- A. A PHYSICAL BARRIER MAY BE APPLIED TO SEPARATE THE ALUMINUM FROM THE P.T. (e.g. FELT PAPER). FACTORY POWDER COATING MUST BE CERTIFIED BY THE MANUFACTURER AS AN ADEQUATE BARRIER.
- B. PAINT ALUMINUM WITH ASPHALT PAINT.C. PAINT ALUMINUM WITH ALKALI RESISTANT BITUMINOUS PAINT.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO COMMENCING WITH CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH MAY NOT BE IN ACCORDANCE WITH DESIGN CONDITIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOB SITE CONSTRUCTION SAFETY. FOR FINISHED FLOOR ELEVATIONS, SLOPES, STEPS AND RECESSES, REFER TO ARCHITECTURAL PLANS. FOR SIZE AND LOCATION OF MECHANICAL SLEEVES AND OPENINGS. REFER TO MECHANICAL AND ARCHITECTURAL PLANS.

TYPICAL WOOD HEADER DETAIL N.T.S.

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